CONCRETE AND CONCRETE AGGREGATES

THESIS

by

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INTRODUCTION

The primary purpose in choosing concrete and concrete aggregates for this thesis was to give the writer a more complete knowledge of these materials, especially for this immediate vicinity.

At the start of this experiment the fixed objective was to determine the relative strengths of some of the important stones that are located in Los Angeles and vicinity. This particular problem was brought to the writer's attention after it was found that Eaton Canyon materials that were used in a recent addition to the Rose Bowl gave concrete that was far inferior to the concrete that was being prepared under the supervision of Mr. Byron Hill at the Institute, these latter materials being secured from San Gabriel. After some investigation the difference in strength of these two concretes were found to be partially due to the aggregates.

After the experiment was started various tests and experiments somewhat naturally followed in succession. Some stones, for example, might be far superior in strength and hardness to stones from another source, yet the grading of these stones might be such as to give a relative weak concrete under some fixed mix. Or, as was also found, certain stones may be preferred under different conditions of use or preparation of the concrete.

This report is on the whole somewhat interrelated and it would hardly be fair to quote too strongly any particular small section of the experiment.

The lack of consistency in certain phases of this experiment has left the writer with a dubious feeling as to the precise validity of certain parts. It is hoped that certain parts of this report shall encourage further work for a check on the results presented here and a continuation of a field of endeavor that has hardly been touched.

Sand and stones were obtained from each of the following 4 sources; Eaton Canyon, San Fernando, San Gabriel, and Catalina -Long Beach. From this last source sand was obtained at Long Beach and stones at Catalina. These 4 sources of materials were chosen because they were considered the most representative for Los Angeles, Pasadena, and vicinity. Concrete sand and both 3/4 inch and $l\frac{1}{2}$ inch crushed rocks were desired. In some cases where 3/4 inch and $l\frac{1}{2}$ inch rock could not be obtained, it was necessary to substitute 1 inch rock for the 3/4 inch and 2 inch rock for the $l\frac{1}{2}$ inch rock. In some cases it was also necessary to substitute gravel for crushed rock.

The Eaton Canyon sand and 1 inch crushed rock were obtained from a Pasadena City construction job, and the $l\frac{1}{2}$ inch gravel was obtained from a bin at the Eaton Canyon plant.

San Fernando sand, 1 inch gravel, and 2 inch crushed rock were obtained from the bins at the San Fernando plant of Graham Bros.

San Gabriel sand, 1 inch gravel, and $l_{\hat{\Sigma}}^{\frac{1}{2}}$ inch gravel were obtained from the bins of Graham Bros.' San Gabriel plant.

The Long Beach sand and Catalina 1 and 2 inch crushed rocks were sent to the Institute through the courtesy of R. W. Livingston who was at the time connected with Graham Bros., Inc.

When all of the materials were on hand, a general geological observation was made with the assistance of David Scharf, a graduate geoldgist at the Institute.

The Eaton Canyon rock had a comparatively large amount of mica which was very cleavable. A good many of the rocks were gneissic, i.e., the minerals were arranged in bands or layers.

The Catalina rock was fine grained and compact with practically no bands or layers. The crushing surfaces were large and very smooth and flat. After several months of weathering the rocks seemed to become somewhat darker on the surfaces.

The San Fernando rock was somewhat less gneissic and micaceous than Eaton Canyon rock but there was, however, a noticible amount present. There were some decomposed peebbles present that could be broken with the hand.

The San Gabriel rock was finer grained and much less gneissic or micaceous than the rocks of Eaton Canyon or San Fernando. It contained a few pebbles of decomposed rock.

The classification in strength, from a geological study of the rocks, is Catalina first, San Gabriel and San Fernando about the same, and Eaton Canyon by far the weakest.

PHYSICAL AND GEOLOGICAL SURVEY

3.

To carry the geological study of the rocks a little further, some rocks from each source were broken and the interior of the rocks were observed. The method of breaking the rocks gave an approximate relative measure of the strength of the four rocks. The priture on page <u>S</u> illustrates the method of breaking. The same testing machine that was used for testing specimens was used but the maximum capacity was reduced to 15,000 pounds and the minimum scale division was then

one pound. The rate at which the cross-head moved was about .05 inches per minute, the same as is required for the standard compressive tests of concrete cylinders and which was used throughout. The individual rock that was to be tested was placed on a three quarter inch board that was laying on the weighing table and the load was applied through a steel bolt head that measured one and one quarter inches across and one half inch thick. The load was applied until the rock cracked. broke. or crushed. The board was used in order to give a large bearing area on the bottom. The rocks that were to be tested had to be carefully selected to be sure that the load was applied by the bolt head on only one flat surface of contact and also that the bottom of the rock had a good resting area directly beneath the point where the load was applied. Approximately eight pounds of æch of the four larger rocks were taken and from these the specimens were chosen. There were three rocks tested from each source as explained above and then three more were tested but instead of the three quarter inch board a 3/4 inch square wood was used.

RESULTS OF BREAKING STRENGTH WITH 3/4 INCH

BOARD BASE

Catalina	Breaking load in pounds	Thickness in inches	Remarks
	2,140	0.5	Dense crystalline structure.
	1,829	1.1	Very fine structure. Partly
			decomposed.
	1,487	1.3	Partly decomposed rock.
			Uniform throughout.
San Fernando	1,706	0.8	Dense. Large crystals.
			large amount quartz.
	1,625	1.0	Noxbanding. Slightly de-
			composed.

San Fernando (Con.)	Breaking Load in pounds	Thickness in inches	Remarke
, st e	1,793	1.2	Cracked at 9,000 #.
			Mica
San Gabriel	1,418	0.7	Dense. Fine structure.
	÷		Mica.
	946	1.0	Mica.
	(1,390	1.1	Supporting block
			failed, not the speci-
			men.)
Eaton Canyon	719	0.6	Fine structure, Mica,
	906	1.0	Granite. Decomposition.
	902	0.8,	Partly decomposed
			throughout.
	REPRESENTATIVE	BREAKING	VALUES

Catalina1,830 Lbs.San Fernando1,710 "San Gabriel1,400 "Eaton Canyon900 "

RESULTS OF BREAKING STRENGTH WITH 3/4 INCH

A DESCRIPTION OF THE PARTY OF T

STRIP AS BASE

Catalina	Breaking load in pounds	Thicknes in Inches	
	2,377	0.7	Compact impalable
			rhyolitic. Apparently
			not decomposed.
	4,993	1.1	Compact impalable
			rhyolitic rock.
			Slightly decomposed
		2	along break.

Catalina (Con.)	Breaking Load in pounds	Thickness in Inches	Geological Report
а ж	4,062	1.3	Same kind of rock. Break along
			a plane of slight decomposition.
Eaton Canyon	961	0.6	Fine grained mica schist, quartz,
			and feldspar. Break almost par-
	•		allel to schistosity.
	650	0.7	Medium grained, roughly branded,
			quartz, feldspar, and mica. De-
			composed condition accounts for
			weakness. Break normal to plane.
•	985	1.6	Quartz and felspar much more than
			mica. Relatively fresh. No
			banding.
San Fernando	6,198	1.0	Very coarse grained quartz, feld-
			spar, and mica. No banding. Very
			fresh.
	1,800	0.8	Coarse grained quartz, feldspar,
			Mica, and a little hornblende.
			No banding. Slightly decomposed.
	1,921	0.7	Coarse grained quartz and felspar.
			No Banding.
San Gabriel	2,430	0.9	Gueissic medium grained. Quartz,
			felspar, and mica arranged in
			bands which are planes of weakness,
			but breal occured almost normal to
			these bands. The load was applied
			parallel to the bands.

San Gabriel (Con.)	Breaking Load In pounds	Thickness in Inches	Geological Report
	2,242	0.9	Medium grained granite, slightly
			altered, toward a gneiss. Much
			more quartz and felspar than mica.
		4	Mica arranged in bands but to a
			much less degree than above rock.
			Part of break across and part along
		e.	plane of mica.
	2,995	0.9	Quartz, felspar, and mica approx-
			imately equal in amount; uniformly
			distributed with flakes of mica
			generally parallel to each other.

REPRESENTATIVE BREAKING VALUES

strength.

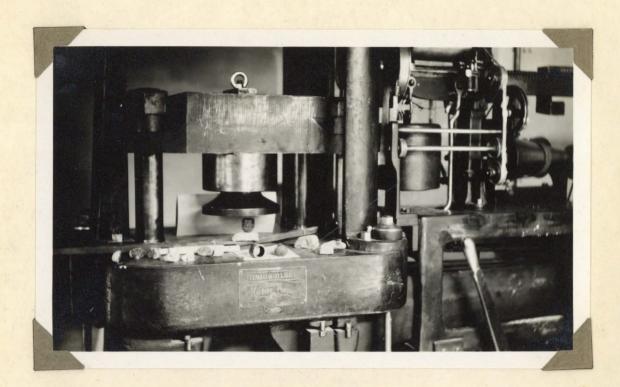
Nox banding. Comparative freshness

of rock and absence of banding or

sebistosity accounts for high

Catalina	3	4,060
San Gabriel		2,430
San Fernando		1,920
Eaton Canyon		96 0

Since there were few suitable specimens from the 8 lbs. of rock samples in each group, those first chosen and used with the flat board test gave better and more representative results as the range indicates.



TESTING PHYSICAL STRENGTH

OF STONES

UNIT WEIGHTS OF MATERIALS

Sand (Dried)

	Wt. of 0.10 cu.ft.	Wt. of l cu.ft.
Eaton Canyon	10.53	105.3
San Fernando	11.36	113.6
San Gabriel	11.03	110.03
Long Beach	10:12	101.2
	Check on above.	
	Wt. of 1/2 cu.ft.	Wt. of 1 cu.ft.
Eaton Canyon	51.2	102.4
San Fernando	56.4	112.8
San Gabriel	54.0	107.9
Long Beach	48.8	97.6

Sands arrayed by weight San Fernando San Gabriel Eaton Canyon Long Beach

,	wt. of $1/2$ cu.ft.	Net Wt. 1 cu.ft.
Eaton Canyon	47.69	95.3
San Fernando	51.45	102.9
San Gabriel	52,77	105.5
Catalina	46.79	93.6
4 ° 3	Arrayed by weight	
(\mathbf{r}_{i+1}^{*})	San Gabriel	· · · ·
	San Fernando	
	Eaton Canyon	

Catalina

1-1/2 INCH STONE

Eaton Canyon

San Fernando

San Gabriel

Catalina

Wt. 1/2 cu.ft.	Wt.	1	cu.	ft.
49.03		98	3.1	
48.0		96	5.0	
53,67	2	107	1.3	
46.82		93	.6	

N. 19 1. 19 1. 19 1. 19

Arrayed by Weight San Gabriel Eaton Canyon San Fernando Catalina

The apparent specifc gravity of the stones from each of the four sources were determined by the standard A. S. T. M. method. Each of the four small groups selected by a representative method were first weight dry, indicated in the following table by A. After 24 hours soaking in water they were again weighed after being blotted dry - B. The soaked rocks were then submerged in water and the net weight is indicated below by C.

	Catalina	Eaton Canyon	San Fernando	San Gabriel
A	1.47	1.04	1.27	1.32
В	1.485	1.05	1.28	1.34
C	.935	.651	.8106	.634
B-C	.550	.399	.469	.506
Specific $\left(\frac{A}{B-C}\right)$ Gravity $\left(\frac{B-C}{B-C}\right)$	2.67	2.605	2.71	2.61

ARRAYED SPECIFIC GRAVITIES

San	Fernando
Cata	alina
San	Gabriel
Eato	on Canyon

PER CENT VOIDS IN 3/4 INCH ROCK

San Fernando	$\frac{2.71 \times 62.4 - 102.9}{2.71 \times 62.4} = 3$	39.1
Catalina	$\frac{2.67 \times 62.4 - 93.6}{2.67 \times 62.4} = 4$	43.85
San Gabriel	$\frac{2.61 \times 62.4 - 105.5}{2.61 \times 62.4} = 3$	35.2
Eaton Canyon	$\frac{2.605 \times 62.4 - 95.3}{2.605 \times 62.4} = 4$	41.4

ARRAYED ACCORDING TO GRADING BY VOIDS

San Gabriel San Fernando Eaton Canyon Catalina

DISINTEGRATION TEST

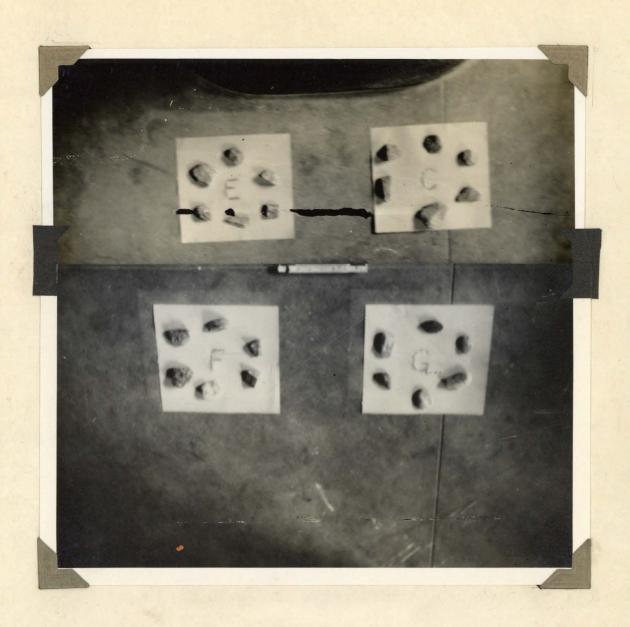
Six lagge rocks were shosen from each of the 4 sources at random. These stones were tested for disintegration by soaking them in a saturated solution of sodium sulphate for about 12 hours and then drying them at about 100 degrees centigrade for the same length of time. All rocks were kept in the same solution and oven and treated exactly the same.

The fault with this test is the personal element in judging the extent of disintegration. Some of the stones cracked in half, others disintegrated to various extents, while some were not even effected. After about 10 complete cycles, the following order gives the resistance to disintegration.

ARRAYED ACEORDING TO RESISTANCE TO WEATHERING.

12.

San Gabriel Catalina San Fernando Eaton Canyon

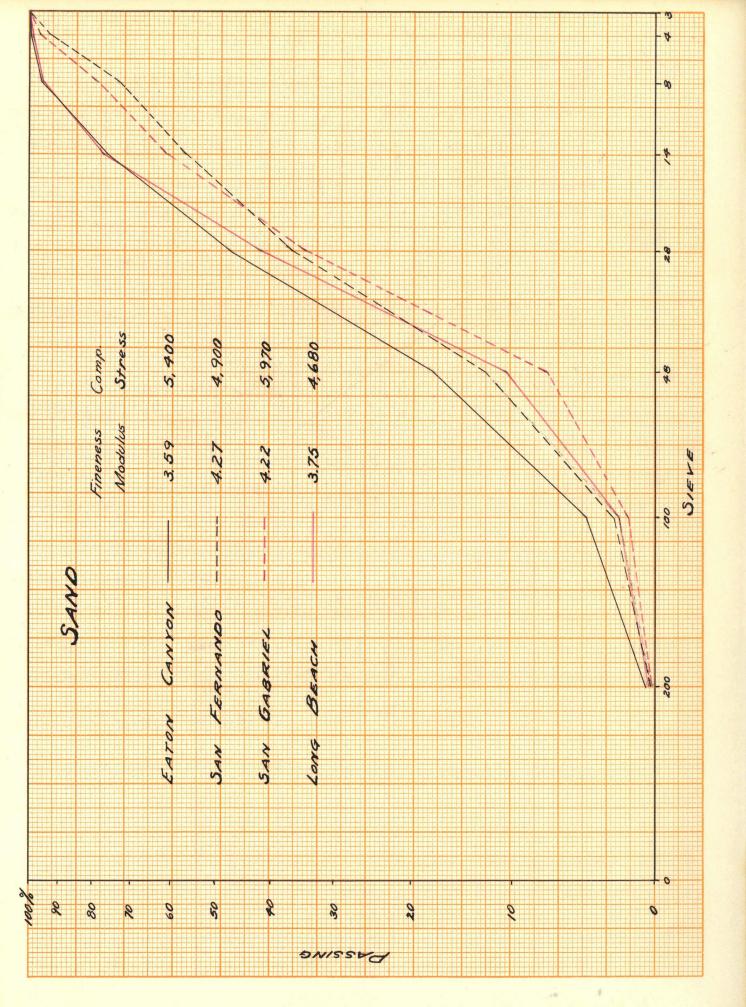


STONES TESTED FOR DISINTEGRATION

SIEVE ANALYSIS OF MATERIALS

Representative samples were secured by quartering about 1/2 cubic foot of each material. The sand was oven dried at about 100° C. The free moisture in the sand was measured but has been omitted because of its lack of importance.

		Sand	Sieve Ar	alysis			
	Sieve		100 48		14	8	4
Eaton Canyon -	Wt. Passing	.02	.14 .58	1.56	2.50	3.14	3.28
	% Passing	.61 4.	25 17.6	47.4	76.	95.5	99.7
Long Beach -	Wt. Passing	.01	.06 .31	1 1.21	2.23	2.73	2.86
n n	% Passing	.35 2	2.08 10.7	41.8	77.1	94.5	98.9
					na da antica ancara anti		
	Wt. Passing	.01	.08 .35	5 1.66	2.94	3.77	4.61
San Gabriel -	% Passing	.21	1.66 7.26	34.5	61.0	72.2	95.6
	u -	•					
			0.0		1 01	0.73	0.04
San Fernando-	Wt. Passing	.01	.08 .4	10 1.16	1.81	2.31	2.94
	% Passing	.31	2.53 12.	52 36.	4 56.7	72.5	92.2

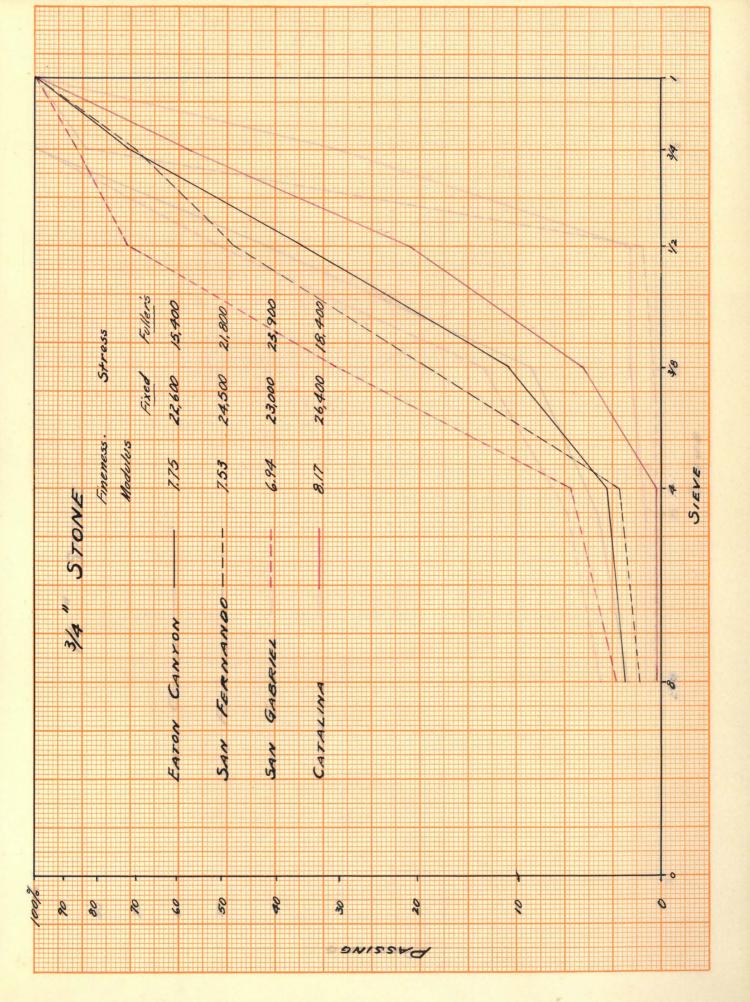


3/4 Inch Stone Sieve Analysis

	Sie ve	8	4	.371	.525	.742	1.05
Eaton Canyon-	Wt. Passing	.11	.16	.53	1.72	3.46	4.8
(Crushed)	% Passing	2,29	3.33	11.0	35.8	72.1	100
~ , v		at					
Catalina	Wt. Passing	.01	.01	.26	1.09	2.95	4.99
(Crushed)	% Passing	.20	.20	5.11	21.4	58.0	98.1
کم ۲ ۱							а. Стали и стали
San Gabriel	Wt. Passing	.14	.30	i. 55	3,66	4.75	5.05
(Gravel)	% Passing	2.77	5.94	30.7	72.5	94.0	100
San Fernando	Wt. Passing	.06	.13	.96	2.39	3.81	5.0
(Gravel)	% Passing	1.2	2.6	19.2	47.8	76.2	100
а 1	1-3	1/2 Ind	ch Sto	one Sie	eve Ana	lys is	a lit
	Sieve	.371	.525	.742	1.05	1.5	2.0
Eaton Canyon-	Wt. Passing	.12	.24	.54	2.50	5.96	n 9 - z
(Gravel)	% Passing	2.0	1 4.02	9.05	41.9	100	
Catalina	Wt. Passing	.01	.01	.02	.02	3.13	10.34
(Crushed)	% Passing	.97	.97	1.93	1.93	30.22	100
San Gabriel	Wt. Passing	. 32	.53	1.12	A. 33	8.45	
	% Passing					100	
(810/01)	// 1000 1 16						
San Fernando	Wt. Passing	.02	. 02	.02	.68	4.93	5.84
(Crushed)	% Passing	.34	.34	• 34	1.17	84.5	100
						679-110-110-1 0-10-1	

16.

4 . * 1



FINENESS MODULUS OF SAND

-			Cent Sa		ing by	Weigh	t	
Sieve	200	100	48	28	14	8	4	5
Opening	.0029	.0058	.0116	.9232	.046	.093	.185	
Eaton Canyon	.61	4.25	17.6	47.4	76.	95.5	99.7	341.06
San Fernando	.31⁺	2.53	12.52	36.4	56.7	72.5	92.2	273.16
San Gabriel	.21	1.66	7.26	34.5	61.0	78.2	95.6	278.43
Long Beach	.35	2.08	10.7	41.8	77.1	94.5	98.9	325.43

F.	INF	ENESS	MODU	LUS	-
Nav	1.0004-0040		$\frac{\partial (x_{i}, x_{i}, y_{i})}{\partial x_{i}} + \frac{\partial (x_{i}, y_{i})}{\partial $	an an a station of a	
7	X	100	· 2		
	******		100	ł.	

Eaton Canyon 3.59 San Fernando 4.27 San Gabriel 4.22 Long Beach 3.75

		FINENESS	
	Fei	rnando	3.27
San		oriel	3.22
Long	B	each	2.75
Eato	n (Canyon	2.59

* In this final grouping the values are unity ; or one; less than above.Since the 200 mesh is commonly omitted these latter units take this into consideration.

CEMENT

The cement that was used was secured from the various construction jobs that were in progress at the Institute during the time of the experiments. All of the cement, which was of the Riverside Portland brand, came from the same sources but in different shipments. To check the consistency in method of testing and strength of the cement, 4 sacks were tested. The neat cement mixtures were all thoroughly mixed by hand in a glazed pan that was approximately 12 inches in diameter and 4 inches deep. For reference, the sacks were numbered. After the specimens were 1 day old they were then placed in the water curing tank in the testing materials laboratory. The temperature of the water in the curing tank was 139 C. Oiled metal forms were used for all the specimens. All compression tests were made on the Tinius Olson Testing Machine with 150,000 lbs. maximum capacity and a minimum scale division of 10 lbs.

CEMENT MIXTURES

Sack #1

Gross Wt. Cement		12.41 Lbs.
Pan	a	1.34
Net	=	11.07 "
Net Vo. Water	-	<u>1273</u> cc.
W/C Ratio		.382 by Volume.

2.86 gal. per cu. ft. Cement.

CEMENT MIXTURES (CON.)

	CEMENI MIAIORES	CON	•)
Sack #2	Gross wt. Cement	-	10.37 Lbs.
	Pan		1.32
,	Net		9.05
	الانترار		
	Net. Vo. Water		<u>1040</u> cc.
	W/C		.382 by volume
			2.86 gal./ c. ft. cement.
Sack #3	Gross Wt. Cement	-	9.33 Lbs.
ar.	Pan		1.33
	Net	*	8.00 *
	Net Vo. Water	-	920 cc.
	Water Cement Ratic) 184	.382 by volume.
	ti ti ti		2.86 gal.
Sack #4	Gross Wt. Cement		9.33 Lbs.
	Pan	-	1.33
	Net	-	<u>8.00</u> **
	Net Vo. Water	-	9.20 cc.
	Water Cement Ratio) #8	3.82 by volume.
	48 58 59	-	2.86 gal.
Combi na ti	on of above four sack	(S .	
	Gross Wt. Cement		12.41 Lbs.
Х ₁₁	Pan	-	1.34
	Net		11.07 "
	Net vol. water	-	1273 cc.
	Water cement ratio) ==	.382 by volume.
	29 85 8 2		2.86 gal.

28 DAY CEMENT TESTS

	Breaking Strength		h Remarks
l [≟] " x 3" Cylinders-Sack#1	26,820	4,050	Sudden breaking of small pieces. Cement dust in center
	22,960		Chipped with first appli- cation of loading.
	20,230	20.450	Sudden failure.
Area = 1.77 Sack#2	16,690	10,450	Chipped and broke across near the bottom.
	6,270		Bottom was untrue. Felt used.
Sack#3	14,690	5,930	80 80 au 80
Sack#4	9,860	8,810	
	21,460		Chipped early and continu- ously; load still increased.
nbination of above	20,980	dan Manang yang ang ang ang ang ang ang ang ang ang	
	21,250		600 cm .ex (60
	14,950		ente que que filip anticipante de la contraction de la contraction anticipante de la contraction de la cont
	Breaking Strength	Average Unit Brkg. Strengt	h <u>Remarks</u>
	46,500		- esa des eve Das
3" x 6" cylinders-Sack#	L	8,330	
	71,330	yungutatus malanzaku yaan masanaku naku antanaku unkurut kutu ya	Little early chipping. Sudden Failure.
	58,150		Sudden Failure
Area = 7.07 Sack#2	3	8,250	
	58,630	ŵn an 1930 1965 19 66 1967 1967 1967 1967 1967 1967 1967 19	Felt Used.
	41.750		Much early chipping.
Sack#3	3	6,500	
	50,000		Started chipping at about 14,000 Lbs.

22

	28 DAY CEMENT TESTS (CO	N。)
	Breaking Average Unit Strength Brkg. Streng	th Remarks
	49,900	Chipping at 26,000 Lbs.
Sack #4	7,370	
Combination	54,230 38, 370 49,930 5,960	Chipping at 36,000 Lbs. First Chipping at 32,000 Lb Ran Cross-head too fast. Chipping at 37,000 Lbs.
e y B.	38,230	Chipped at 33,000 Lbs. and 38,000 Lbs.
* 25	Breaking Lead Unit Stren	ngth
2" Cubes Sack #1	20,250 7,560.	
Area • 4 Sack #2	32,080 8,010	
Sack #3	24,650 6,140	an a stag a s
Sack #4	19,500 4,880	s
Combination	14,600 3,650	
	Briquetts	e en
Sack #1	800	
Sack #2	634	n 1995 - San
Sack #3	555	
Sack #4	855	
Combination	478	

Exactly 1000 c.c. (2.2 Lbs.) of water and 4.7 lbs. (0.05 cu. ft.) of cement were added to 0.10 cu. ft. of each of the 4 sands. All sands were room dried. When the specimens were 27 days old they were removed from Institute Laboratory curing tank. The specimens were tested from 18 to 23 hours after being removed from the curing tank. The temperature of the curing water was approximately 13° C. during the curing period. Since all metal forms were used, the folliwung areas represent the differnet specimens: the $1\frac{1}{2}$ " x 3" molds area - 2.01; the 2" cubes area - 4.0; the 3" x 6" cylinders area - 7.09; and unity for the briquettes.

EATON CANYON

	1 2	" x 3" C	ylinders		2" Cubes	an a
Breaking Force	8,340	7,050	10,340	27,730	26,700	26,080
Unit Max. Stress	4,150	3,510	5,150	6,930	6,680	6,520
Average(Arith)		4,270			6,710	
Median		4,150			6,680	

Breaking Force	3" x6" Cylinders	Briquettes
Unit Max. Stress	39,290 33,960 37,620 5,550 4,790 5,310	435
Arith Average	5,220	
Median	5,310	

SAN FERNANDO

lģ" x	3" Cylinders	2" Cubes
Breaking Force 7,140	4,820 9,810	23,800 27,480 19,260
Unit Max. Stress 3,560	2,400 4,880	5,950 6 ,870 4,810
Arith.Average	3,610	5,880
Median	3,560	5,950

	<u>3" x 6</u>	" Cylind	ers	Bi	riquettes	
Breaking Force	38,290	-	38,770		445	
Unit Max. Stress	3 5,400		5,460		a a di s	¥
Arith.Average	¥(я) — я т	5,430	**************************************			
Median	3					

SAN GABRIEL

F e	<u> </u>	x 3" Cyli	nders	1111. -1111111111111111.	2" Cuber	3
Breaking Force	9,420	10,520	9,370		25,660	25,460
Unit Max. Stress	4,680	5,230	4,660	+	6,420	6,370
Arith. Average		4,860			6,390	
Med lan		4,680		3	-	
	3"	x 6" Cylir	nders	an state to provide a state of the state of	Briquette	8
Breaking Force	39,660	38,160	42,330		390	
Unit Max. Stress	5,600	5,380	5,970			

 $\frac{x}{\langle \theta_{1}(s)\rangle},$

25.

5,650

Nedian 5,600

Arith. Average

LONG BEACH

	14" x #2 Cy1	inders		2" Cubes	
Breaking Force	9,520 9,000	6,530	18,200	21,100	23,030
Unit Max. Stress 4	4,730 4,480	3,250	4,550	5,280	5,760
Arith. Average	4,150			5,200	
Median	4,480	÷		5,280	and the

Breaking Force 3"	x 6" Cylinders	Briquettes
34,520	34,780 31,530	-
Unit Max. Stress 4,870	4,900 4,450	
Arith. Average	4,740	
Median	4,870	* *

RESULTS OF COMPRESSIVE TESTS FOR

alon is sure that we have the second and the second and the second second second second second second second s

CEMENT-SAND SPECIMENS

(Cylinders and cubes weighed alike)

	Arith. Averages	Arith. Average of Medians.
Eaton Canyon	5,400	5,380
San Fernando	4,900	4,980
San Gabriel	5,970	5,560
Long Beach	4,680	4,880

ARRAYED SAND TEST RESULTS

San Gabriel Eaton Canyon San Fernando

Long Beach

CONCRETE SPECIMENS

Two series of cylinders were prepared and tested. One of the series was where the aggregate ratios were fixed, and the other was where the aggregate ratios for each set of materials were determined by use of Fuller's Curve for maximum density.

MIXES WITH FIXED RATIOS

For the mixes with the fixed ratios an attempt was made to make the break go through the rock, the purpose being to get the relative strength of the rocks. For this reason a rocky mix was chosen. The water, cement, silt, and sand were constant for all of the mixes. San Gabriel silt and sand were used. The silt was the only aggregate with water in it. Since this was such a small quanity and constant for all mixes, it was neglected.

All concrete for the fixed ratio specimens were mixed by hand in the Institute Testing Laboratory. The mixing was done in a large metal mixing box, which was cleaned and dampened for each individual mix. The materials were first thoroughly mixed dry and then the water was carefully added and again the mixture was thoroughly mixed.

The specimens were numbered for each set. Number one was where the specimen form was of cardboard with a tin bottom. The cardboard had only a very light parafin coat. Number two, three, and four were cardboard forms that were well parafined throughout. Number two had a machined metal plate placed in its bottom to save later capping on

one end. Specimen number five was of metal It was, immediately after preparation, placed in the Ro-Tap Machine that is used for shaking sieves for twenty two complete shakes or about ten seconds. About one half inch of concrete was shaken out of the form. The water lost or brought to the top was negligable.

	CONSTANTS FOR SPECIMENS			
			STONE.	
	Water	Cement	Silt	Sand
Wt. in Lbs.	16.5	29,6	15.6	35.0
Volume in cu.ft.	.264	.315	.157	.324
Ratio by Vol.	.838	1.0	0.50	1.03
· · · · · · · · · · · · · · · · · · ·	· > *			1

WT. PER CUBIC FOOT OF CONSTANTS

Water	Cement	S.G. Silt	S.G. Sand
62.5	94.0	99.2	107.9

3/4 INCH STONES

	<u>Catalina</u>	Eaton Canyon	San Fernando	San <u>Gabriel</u>	
Wt. in Lbs.	88.5	90.0	97.2	99.7	а.
Vol. in cu. Ft.	.945	.945	.945	.945	
Ratio by Vol.	3.0	3.0	3.0	3.0	
	Mix (by v	volume) 1.0	0 : 0.50	1.03 2	3.0
	W/C = 6.2	9 U.S. gall	Lons per cu.	ft. cement	
	• 0.84 by	volume.			

	Catalina	Ea ton Canyon	San Fernando	San <u>Gabriel</u>
Type of Stone	Crushed	Crushed	Gravel	Gravel
Wt. per cu.ft.	93.6	95.3	102.9	105.5
Flow	173	210	235	230
Slump	312	5幸	7쿨	81

MIXES WITH BOTH SIZES OF STONES

	Water	Cement	silt	Sand
Wt. in Lbs.	9.9	17.76	9.36	21.0
Vol. in cu. Ft.	,1585	.189	0943	1944
Ratio by Vol.	.838	1.0	. 50	1.03

WT. PER CUBIC FOOT OF CONSTANTS

Water	Cement	S.G. Silt	S.G. Sand
62.5	94.0	99.2	107.9

3/4 INCH STONES

		A second se	CALENDARY STREET, SALAR STREET, SALAR STREET, STRE	
	<u>Cataline</u>	Eaton Canyon	San Fernando	San <u>Gabriel</u>
Wt. in Lbs.	17.7	18.0	19.5	20.0
Vol. in cu. Ft.	.189	.189	.189	.189
Ratio by Vol.	1.0	1.0	1.0	1.0

1-1/2 INCH STONES

k sa ka k	<u>Catalina</u>	Eaton Canyon	San Fernando	San <u>Gabriel</u>
Vol. in Lbs.	35.4	37.1	36.3	40.6
Vol. in cu. ft.	. 378	.378	.378	.378
Ratio by Vol.	2.0	2.0	2.0	2.0

Mix (by volume) 1.0 : 0.50 : 1.03 : 1.0 : 2.0 W/C • 6.29 U.S. gal. per cu. Ft. cement.

Ca	talina	Eaton Canyon	San Fernando	San Gabriel
Type of Large Stone	Crushe d	Gravel	Crushed	Gravel
Wt. of Large Rock	93.6	98.1	96.0	107.3
Flow	•	230	230	230) Gravel 260) Water
Slump	7출	72	62	5 <u>1</u>
Remarks Very	wet.	wet.	wet.	Wettest. Separation of aggregates.

28 DAY COMPRESSION TEST

San Gabriel

3/4" Stone

- 1. 61.200 Break thru very few rock.
- 2. 60,300 Very few breaks. Decomposition.
- 3. 56,400 Practically no rock breaks.
- 4. 55,800 " " "
- 5. 63,200 Few Breaks.

3/4" and 1-1/2" Stone

1. Less than 20,000 Lbs. Poor Matrix. Cement washed out.

28 DAY COMPRESSION TEST (CON.)

3/4" and 1-1/2" Stone

2. Less than 20,000 Lbs.

3. Less than 20,000. Mocks loose. No. rock breaks. Poor matrix.

Note: Poor bond thru-out.

Eaton Canyon 3/4" Stone

- 1. 62,500 Many rock breaks. Much mica exposed.
- 2. 51,000 Poor bearing surface.
- 3. 54,700 Break thru several rocks.
- 4. 66,100 Break thru still fewer rocks.
- 5. 58,900 Many rock breaks. Much mica exposed.

3/4" and 1-1/2" Stone

1.	50,600	Break	thru	several	rocks:	around	big	ones.
2.	46,500	13	Ħ	9t	54	88	ŧł	18
3.	52,600	17	53	**	15	23	Ħ	19

Note: Many rocks broken.

San Fernando

31.

3/4" Stone

- 57,700 Break thru few rocks. Mica and decomposition.
 62,700 Same.
 65,000 "
- 4. 64,500 "
- 5. 65,700 *

28 DAY COMPRESSION TEST (CON.)

San Fernando

3/4" and 1-1/2" Stone

- 1. 47,900 Break thru few. Mica and decomposition.
- 2. 59,000 Break thru few. Mica only.
- 3. 39,500 Break thru several. Note: Mostly mica. Some coarse
 grained rocks.

Note: Poor bond. Very few rock breaks.

Catalina

3/4" Stone

- 1. 65,500 Break thru several rocks. Rocks showed decomposition.
- 2. 73,200 Few breaks. Mostly decomposed rocks.
- 3. 70,000 Break thru many rocks. While rock looks strong and fine grained and light in color, the breaks were dark and coarse grained. Few of the breaks looked strong like the exterior.

4. 69,600 Break thru several rocks. Rock showed decomposition.
5. 61,000 Break thru few rocks. About 1/2 decomposed. Note: A specimen cracked at damp top at 50,000. Broken upper half.

3/4" and 1-1/2" Stone

32

 43,400 Broke thru several big rocks and several small ones.
 Some of the big rocks appeared decomposed on the interior only. Note: Cracked at 42,000.

28 DAY COMPRESSION TEST (CON.)

Catalina

3/4" and 1-1/2" Stone

2. 39,200 Broke thru and around several large rocks. Broke thru few small ones.

3. 36,300 Broke thru several big rocks. Some of the broken rocks looked strong while most of the breaks showed decomposition.

RESULTS OF 28 DAY COMPRESSION TESTS

FOR MIXES WITH WATER, CEMENT, AND SANDS FIXED.

3/	4.8	Stone	Specimens.
8 mi - 1 e	(1.8.1) (1.9.1) ¹ (1.7.1)	and Consecution of the second	معق معمور تهدي الراب بالرابية الأربان الرابي الرابعة المحاصر الأمام

	Eaton	San	San	Catalina
	Ganyon	Fernando	Gabriel	
1	62,500	57, 700	61,200	65,500
2	51:000	62,700	56-400	73, 200
3	54,700	65,000	60,100	70,000
4	66100	64, 500	63:200	69,600
5	58,900	65,700	55,800	61;000
Arith. Aver.	58,600	63,100	59 300	67,900
Median	58,900	64, 500	60,100	69\$600
Aver. Stress	•	24,500	23,0000	26\$400
	#######\$##############################	12 million and a stand and a second secon	来了这些事件,在14日的考虑是是一种的的是不是是这些不是不是是一个的。 在1995年,在14日的考虑是是一种的的是不是是这些是不是是是一个的。	No. Company

3/4" and 1-1/2" Stone Specimens.

	Eaton	San	San	Catalina
	Canyon	Fernando	Gabriel	
1	50,600	47,900	No cement.	39,200
2	46,500	59,000	Specimens	31,700
3	52, 600	39,500	failed be-	36,300
4	-	-	low 20;000	44,000
5		-		43,400
Arith. Aver.	49,900	48, 800		38,900
Median	50\$600	475900		39,200
Aver. Stress	195400	20, 500		15,000

ORDER IN MAGNITUDE OF STRENGTH

FOR SPECIMENS WITH 3/4" STONES.

Catalina	(Crushed stone. Flat surfaces)
San Fernando	(Gravel)
San Gabriel	(Gravel)
Eaton Canyon	(Crushed stone)

ORDER IN MAGNITUDE OF STRENGTH

FOR	SPECIMENS WITH 3/4"	AND 1-1/2" STONES.
4	Eaton Canyon	(Gravel)
	San Fernando	(Crushed stone)
	Catalina	(Grushed stone. Flat surfaces)
	San Gabriel	(Gravel)

ORDER IN MAGNITUDE OF TESTED SPECIMENS

WITH GREATEST NUMBER OF BROKEN STONES.

Eaton Calnyon

Catalina

San Fernando - San gabriel

35

ORDER IN MAGNITUDE OF TESTED SPECIMENS

WITH BEST STONE BOND.

Eaton Canyon

Catalina (3/4")

San Fernando

San Gabriel

MIXES DETERMINED BY FULLER'S CURVES

For the mixes determined by Fullers' Curves an attempt was made to get the most practical mix for the field with each of the four sets of materials. The water and cement were kept constant for all mixes.

All of the concrete in this test was mixed by means of a small gasoline mixer, the same as is very commonly used on small construction work. The drum was three feet in diameter and had three sets of paddles for mixing. It revolved at a rate of about eighteen revolutions per minute, its peripheral speed being about 170 feet per minute.All mixes were mixed for two minutes. The concrete was dumped from the mixer into a wheelbarrow. The concrete was placed in the specimen forms according to A.S.T.M. One man placed concrete by thirds in each of the forms while a second man followed up with the tamping.

The day after preparation the specimens were stripped. The specimens were prepared and cured on the Institute campus beneath one of the large oak trees. The that three days following the preparation of the specimens they were lightly sprinkled each afternoon.

The following.mixes were finally decided upon as the most practical. These mixes were chosen after much labor and with the assistance of Byron Hill who was at the time connected with the Institute Civil Engineering Department.

36

SHOWING A MIXER USED PLACE OF CURING FINISHED SPECIMENS

SPECIMENS PREPARED

ACCORDING TO FULLER'S CURVES & METHODS

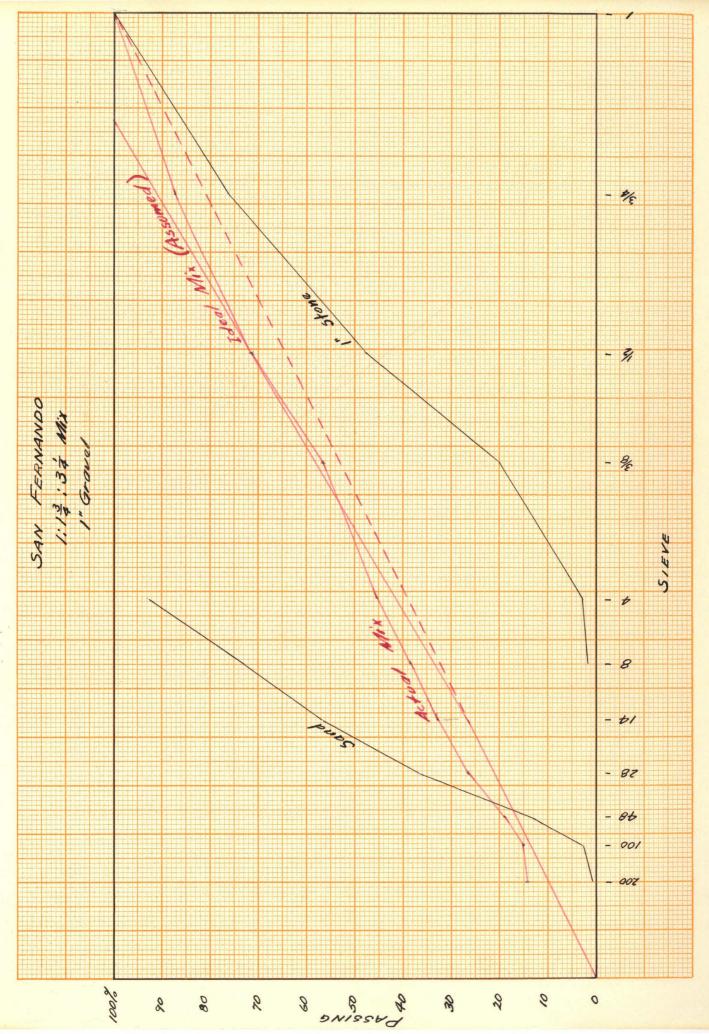


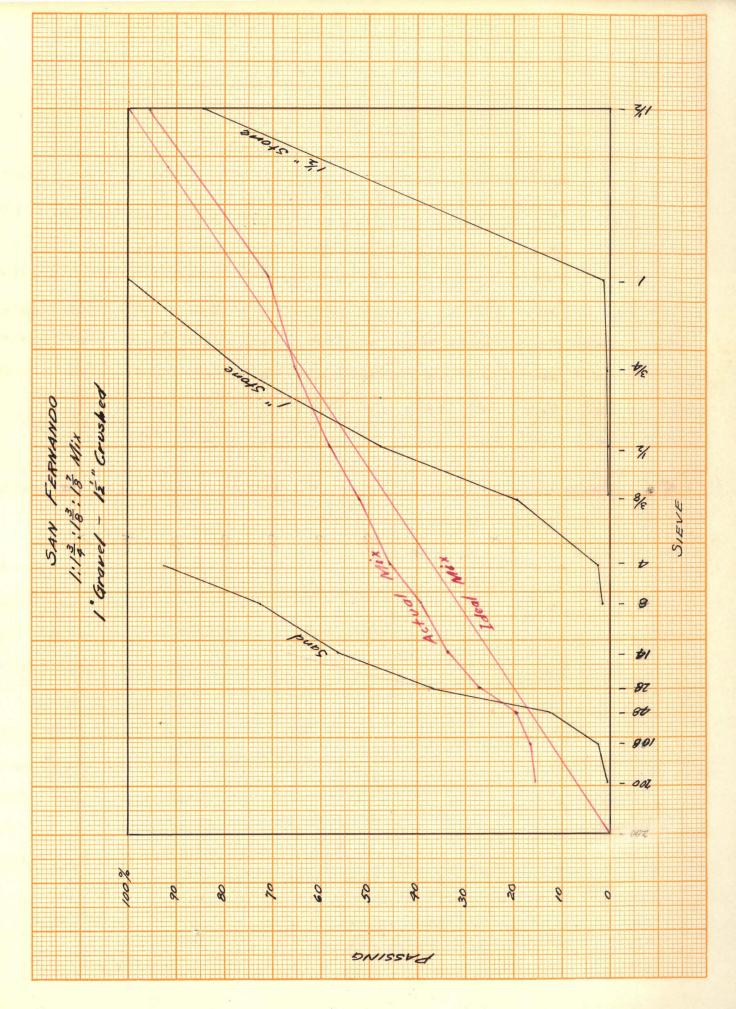
San Fernando Materials

9		Volume				weight			20	Weigh	t		
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Sand 13			3	198 31.6									
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ro tal							626			100.0			
5ie ve	200	10	0 48	28	1	4	8	4 3	<u>3/8</u>	1/2	3/4	_ 1	
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	0.	1 0.	8 4.0	11.5	17.9	9 22	9 29	.2 31	L.6	31.6	31.6	31.	6
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Sand			그릏				198		×	32.3			
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1늘" Ste	one		1 7/	<u>B</u>			180			29.4			
							613			100.0			
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	0.1	0.8	4.1	11.8	18.3	23.4	29.9	32.3	32.3	32.3	32.3	32.3	32,3
	0	0	0	0	0	0.3	0.6	4.5	11.0	17.5	23.0	23.0	23.0
*	0	0	0	0	0		0	0.1	0.1	0.1	0.3	25.0	<u>29_5</u>
	15.4	16 .1	19.4	27.1	33.6	39.0	45.8	52.2	58 .7	65.2	70.9	95.6	100.1

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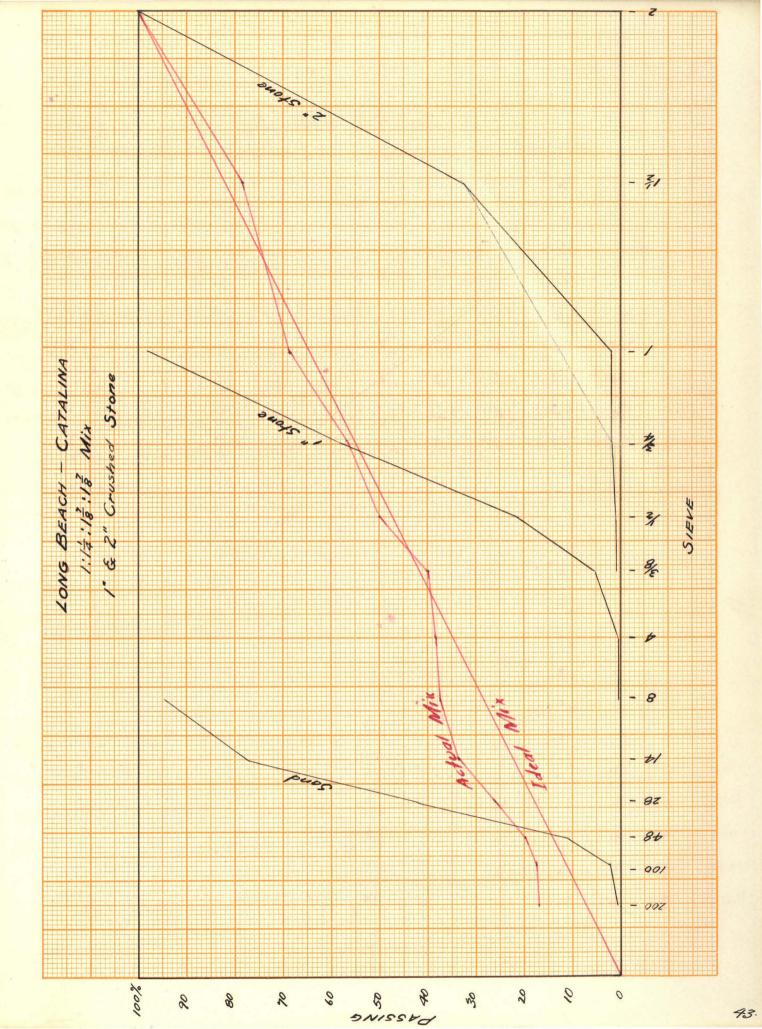




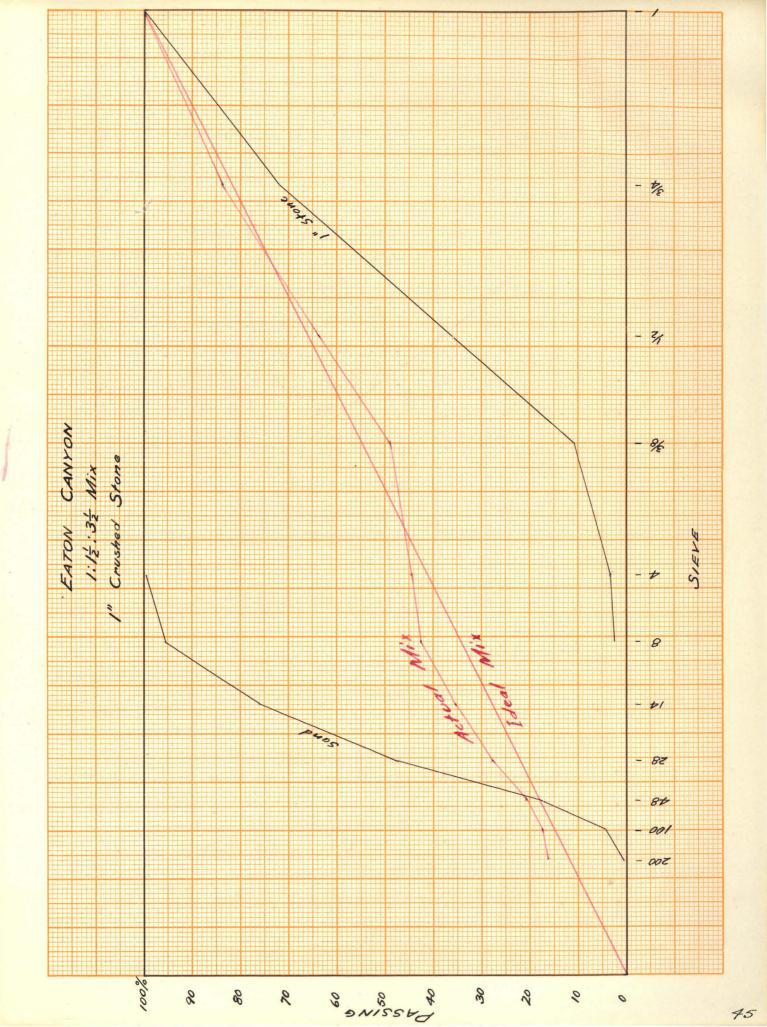
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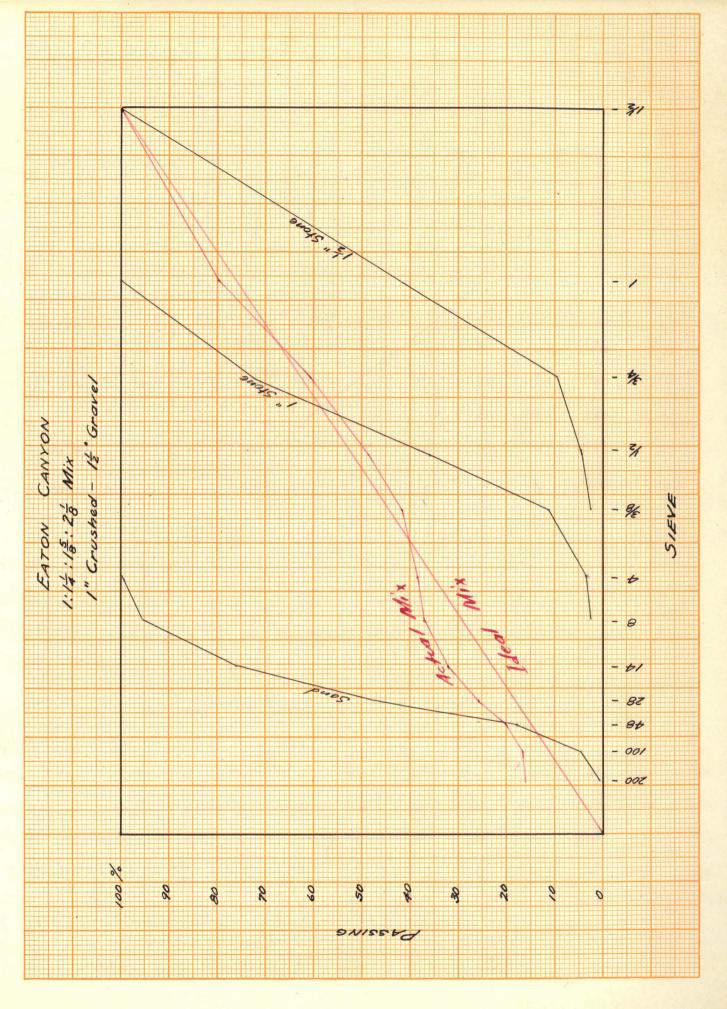
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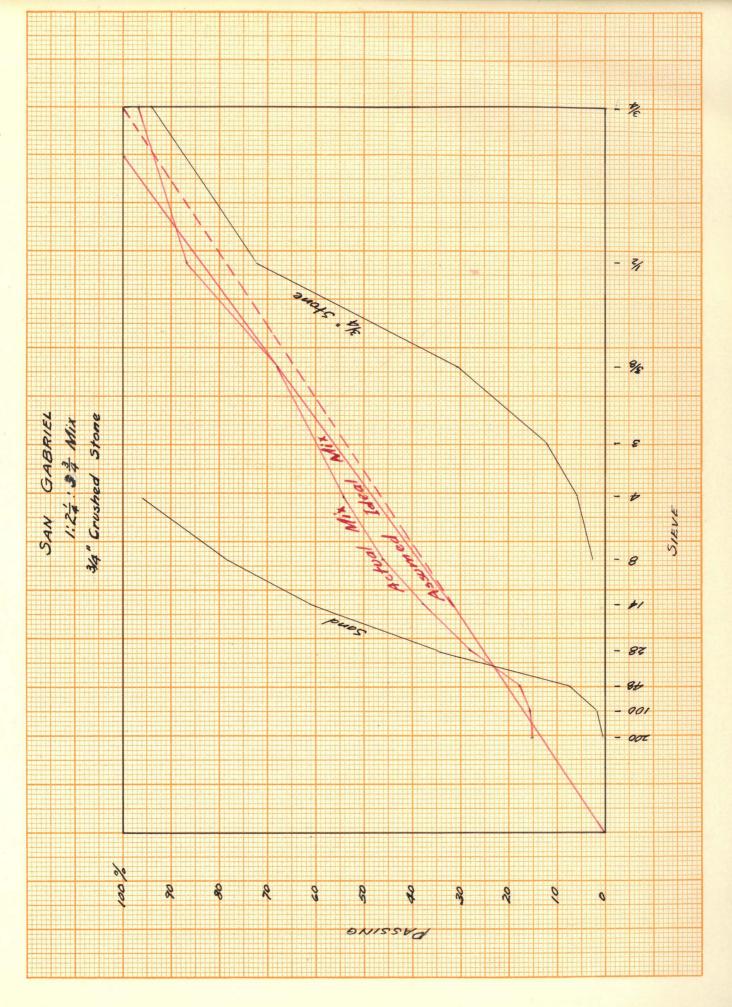


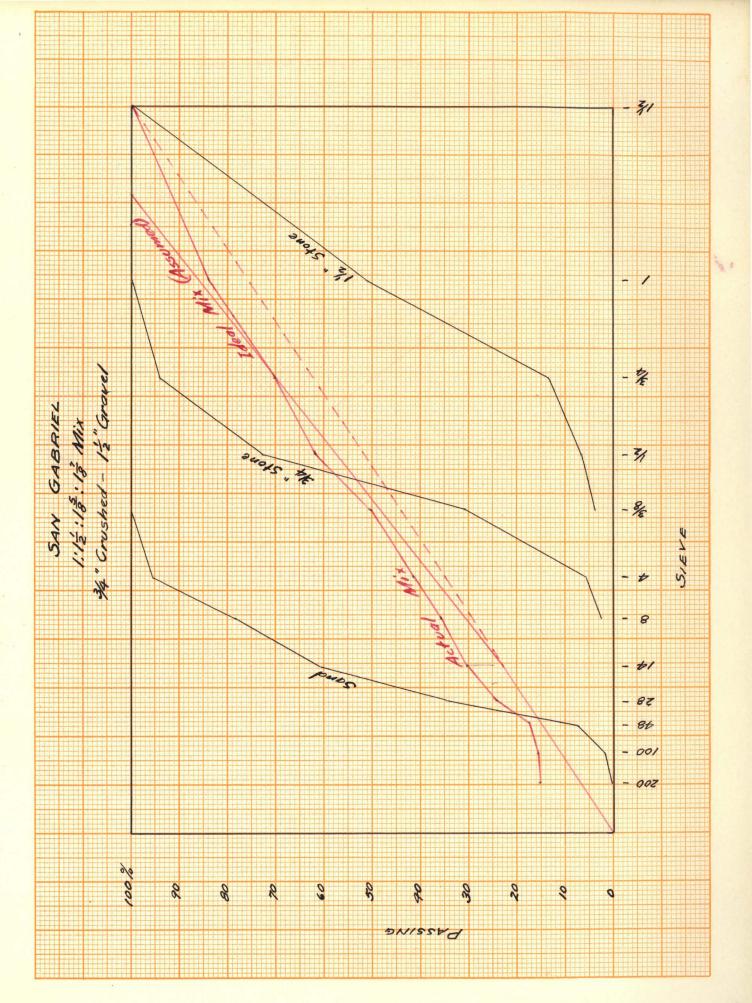
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3/4" S	tone		23			290		48	8.6			
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Sieve	200	100	48	28	14	8	4	3/8	1/2	3/4	1	1
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Sand	0.1	0 0.7	2.8	13.3	23.4	30.0	36.8	38.3	38.3	38.3	38.	3
3/4"	0	0	0	0	0	1.3	2.7	14.4	33.9	43.8	48.	<u>6</u>
	15.2	15.8	17.9	28.4	38.5	46.4	54.6	67.8	87.3	97.2	100.	0
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Sand 3/4" S 1 1/2" <u>Sieve</u>	tone Stone 200	100	1 1 1 1	1/2 5/8 7/8 28	<u>14</u>	94 162 171 <u>201</u> 628 <u>8</u>	4 2	11 21 27 <u>32</u> 100 3/8	5.0 5.8 7.2 2.0 0.0 <u>1/2</u>	3/4		<u>1</u> 출
Sand 3/4" S 1 1/2" <u>Sieve</u> Cem.	tone Stome <u>200</u> 15.0	<u>100</u> 15.0	1 1 1 <u>48</u> 0 15.0	1/2 5/8 7/8 <u>28</u> 15.0 1	<u>14</u> 15.0]	94 162 171 <u>201</u> 628 <u>8</u> 15.0	<u>4</u> 2	11 21 27 <u>37</u> 100 <u>3/8</u> 15.0	5.0 5.8 7.2 2.0 0.0 <u>1/2</u> 15.0	15.0]	L5.0	15.0
Sand 3/4" S 1 1/2" <u>Sieve</u> Cem. Sand	tone Stome <u>200</u> 15.0 0.1	<u>100</u> 15.0 0.4	1 1 1 1 <u>48</u> C 15.0 1.9	1/2 5/8 7/8 <u>28</u> 15.0 1 8.9 1	<u>14</u> 15.0]	94 162 171 <u>201</u> 628 <u>8</u> 15.0	<u>4</u> 2 15.0] 24.7 2	11 21 27 <u>34</u> 100 <u>3/8</u> 15.0 1 25.8 2	5.0 5.8 7.2 2.0 0.0 <u>1/2</u> 15.0	15.0 3 25.8 2	L5.0 25.8	15.0 25.8
Sand 3/4" S 1 1/2" <u>Sieve</u> Cem. Sand 3/4"	tone Stome <u>200</u> 15.0 0.1 0	<u>100</u> 15.0 0.4 0	1 1 1 1 1 1 1 1 1 9 0	1/2 5/8 7/8 <u>28</u> 15.0 1 8.9 1	<u>14</u> 15.0] 15.7 2 0	94 162 171 <u>201</u> 628 <u>8</u> 15.0 20.2 20.2 20.7	4 2 15.0 1 24.7 2 1.6	11 21 27 <u>34</u> 100 <u>3/8</u> 15.0 1 25.8 2 8.4 1	5.0 5.8 7.2 2.0 0.0 <u>1/2</u> 15.0 25.8 19.7	15.0 3 25.8 2 25.6 2	15.0 25.8 27.2	15.0 25.8 27.2
Sand 3/4" S 1 1/2" <u>Sieve</u> Cem. Sand 3/4" 1 1/2"	tone Stome 200 15.0 0.1 0 0	<u>100</u> 15.0 0.4 0	1 1 1 1 1 1 1 1 9 0 0	1/2 5/8 7/8 <u>28</u> 15.0 1 8.9 1 0	14 15.0 1 15.7 2 0	94 162 171 <u>201</u> 628 <u>8</u> 15.0 1 20.2 2 0.7 0	4 2 15.0 1 24.7 2 1.6 0	11 21 27 27 27 27 27 27 27 27 27 27 27 27 27	5.0 5.8 7.2 2.0 0.0 1/2 15.0 25.8 19.7 2.0	15.0 3 25.8 2 25.6 2 <u>4.2</u> 3	L5.0 25.8 27.2 L <u>6.4</u>	15.0 25.8 27.2 <u>32.0</u>





RESULTS OF 28 DAY COMPRESSION TESTS

FOR MIXES DETERMINED BY FULLUR'S CURVES.

3	14"	stone	Sne	ai.	mans	
~	-20	maatto	who	01	THE TTE	•

	Eaton Canyon	San Fernando	San Gariel	<u>Catalina</u>
1	36,000	57, 600	64, 900	43,800
2	44, 300	46,000	67, 300	47,900
3	395700	63,000	74,700	50\$400
Arith. Aver.	40,000	55,500	69,000	47,400
Median	39,700	57,600	67,300	47,900
Aver. Stress	15,400	21,800	25,900	18;400

3/4" and 1-1/2" Stone Specimens.

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	Eaton Canyon	San Fernando	San Gabriel	<u>Catalina</u>
1.	35,500	40,800	43,700	43,800
2	41,600	48,100	37,900	47,900
3	37,600	45,200	37,100	50,400
Arith. Aver.	38,200	44,700	39% 600	47,400
<u>Median</u>	37 600	45,200	375900	47,900
Aver. Stress.	14,600	17,300	14,900	18,400



TESTED SPECIMENS PREPARED ACCORDING TO FULLER'S CURVES

TABLE OF RESULTS

Notation: CatalinaCat. Long Beach.....L.B. San Gabriel.....S.G. San Fernando.....S.F. Eaton CanyonE.C.

Note: The water-cement ratio of all concrete specimens was 0.84 by volume.

Units used are pounds and inches.

1 2	1	2	3	4
Unit Weights of Sands	S.F. 112.8	S.G. 107.9	E.C. 102.4	L.B. 97.6
Fineness Modulii of Sands	S.F. 3.27	S.G. 3.22	L.B. 2.75	E.C. 2.59
Unit Strengths of Cement-Sand Specimens	5560	E.C. 5380	S.F. 4980	L.B. 4880
Unit Weights of 3/4" Stones	S.G. 105.5	S.F. 102.9	E.C. 95.3	Cat. 93.6
Unit Weights of 1-1/2" Stones			S.F. 96.0	
Specific Gravities of Stones	S.F. 2.71	Cat. 2.67	S.G. 2.61	E.C. 2.605
Per Cent Voids in 3/4" Stones	S.G. 35.2	S.F. 39.1	E.C. 41.4	Cat. 43.85
Resistance to Weathering	s.c. s.G.		S.F.	
Geological Classification of Stones	Cat.	S.G.	S.F.	E.C.
Physical Strengths of Stones	Cat. 1830		1400°	E.C. 900

Unit Strength of Concrete Specimens

		1	2	3	4
Unit Strength of	Concrete Specimens				
3/4" Stones	1.0;0.5;1.03;3.0			S.G. 23,000	E.C. 22,600
	Fuller's Curves	S.G. 25,900	S.F. 21,800	Cat. 18,400	E.C. 15,400
Both Stones (3/4",1-1/2")	1.0;0.5;1.03;1.0;2.0			Cat. 15,000	S.G. Not Rec.
	Fuller's Curves	Cat. 18,400	S.F. 17,300	S.G. 14,900	E.C. 14,600
Flow of Concrete	for 3/4" Stones				
	Fixed Ratio	S.F. 235	S.G. 230	E.C. 210	Cat. 173
	Fuller's Curves	S.G. 225	S.F. 220	Cat. 210	E.C. 200
Slump of Concrete	for 3/4" Stones			198 - F	
Contras Av outle	Fixed Ratio	S.G. 812	S.F. 71	E.C. 54	Cat.
	Fuller's Curves	8.G. 72	5.F. 52	Cat.	<u>B.C.</u>
Tested Specimens	with Least Broken Stones.	.S.G.	S.F.	Dat.	E.C.
	with Least Loose but Unbar		Cat.	S.F.	S.G.

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PRALIMINARY DISSCUSION.

The results given in this report should not be accepted too fully for several reasons. First, the results indicate that the accuracy was not very good. For example, in determining the unit weights of the sands with 2 different sized measuring devices there was a maximum variation of 3.6%. Incidentally, this variation was consistent throughout the 4 sets of measurements. Second, and most of important of all is the fact that the materials at the bins vary greatly from time to time. The writer made a sieve analysis of the San Gabriel 3/4" stone at 3 different times and the grading varied about as much as any 3 of the different 3/4" stones listed in this report. It can easily be seen how the grading can vary even in a given bin. Likewise, but not to such an extent, there is the probability in variation in strength and other characteristics of a material from a given source. Because of these possible variations the results of such experiments as here recorded can only be fully accepted after a series of duplicate experiments had been conducted at different periods.

Throughout this experiment an attempt was made to secure only comparative values. For this reason the author has felt justified in diverging from common practice or the A.S.T.M. standards. For example, the finess moduluses here recorded are not standard because the Tyler standard set of sievs were not available, nevertheless the results are suitable for the purpose of this report. Then there are the empirical scales used for some of the curves, the purpose being to better illustrate the results. The water-cement ratio has been too high throughout and the curing has been very poor, yet the order and magnitude of the results are satisfactory except in a few instances were concrete was prepared with 1-1/2" stone.

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DISSCUSION.

The original test used for measuring the hardness of stones by crushing them individually on the testing machine is apparently a good method of measuring the hardness of stones. It would be interesting to check the results of a series of tests made on different stones by this method and by the standard rattler test. Obviously it is necessary to test 3 or more individual stones and use as the representive value the median rather than the arithmetic average. Incidentally, it is interesting to note the almost negligible variation between the median and the arithmetic average for the breaking stresses of the concrete specimens. The median is favored over the arithmetic average because 1 freak specimen does not throw the final result off.

The primary object of this experiment was to measure the relative strengths of the different stones here presented. When the concrete specimens that were arbitrarily designed with fixed ratios were tested the stones were graded according to their number of broken surfaces and the bond to the matrix. The order in magnitude of broken stones depends upon the strength of the matrix and its adhesion to the stones and the strength of the stones. The adhesion of the matrix to the stones is dependent upon the porosity of the stones(considering here dried stones) and the roughness of the surface of the stones. The line of failure in a concrete specimen may occur through the matrix, through the stones, along the surface of the stones, as is most generally the case, through a combination of the above 3 possibilities The specimens with the 1-1/2" stones gave low strengths because

of the weakness of the matrix itself and the poor bond between the

matrix and the stones. The water cement was the same as the water and cement for the specimens with the 3/4" stones. In the latter specimens, however, more water was used by the stones for absorption and the dampening of the surfaces since there was more stone surfaces than where the 1-1/2" stones were used. The matrix was weak in the specimens with the large stone because the cement was washed out.

In the mixes with the smaller stone if the break occured along the surfaces of the stones the expected order in strength for the conorete specimens would be:1. Eaton Canyon: 2. Catalina: 3. San Fernando: 4. San Gabriel; this being the order of adhesion based on roughness. With regards to the surface the rocks may be classed in the following manner: Eaton Canyon rock was subangular stream gravel mixed with angular crushed rock; Catalina was irregular crushed gravel with flat surfaces; San Fernando rock was sub-angular stream gravel; and the San Gabriel stone was sub-angular and sub-rounded stream gravel.

If the break was through the rock the brder given in the above paragraph would not necessarily hold but would probably hold if the stones were of the same strength and all other factors, except the smoothness of the surfaces, were the same. From actual observation the order for magnitude of number of broken stones in the tested specimens was: 1, Eaton Canyon: 2, Catalina: 3, San Fernando: 4, San Gabriel. This is the same order as the degree of surface roughness as given above, which indicates that the number of broken stones varies directly as the surface roughness of the stones.

A brief and complete summary of the geological survey of the stones is given on page #3 of this report.

It is interesting to note that the geological classification of the stones, which was made with no knowledge of the sources of the stones at the time, and the rock hardness test that was devised by

the writer as a substitute for the rattler test because of the lack of equipment, gave exactly the same results. Also, this same order was obtained from the concrete specimens that were prepared according to the fixed ratio of 1.0: 0.50: 1.03: 3.00 by volume. Portland cement, San Gabriel silt, San Gabriel sand, and the $3/4^{"}$ stones were used. The water-cement ratio was 0.84 by volume.

The Eaton Canyon stone proved to be the weakest in the hardness test and the poorest from a geological standpoint. It also gave the weakest concrete in both the fixed ratio mixes and the mixes determined by Fuller's curves. Its specific gravity and its resistance to weathering were also the lowest. In the broken concrete specimens the Eaton Canyon stones had more breaks than the stones of any other source. Obviously there is no question but what the Eaton Canyon stone is inferior to the others in all respects.

The Catalina stone, which was the hardest and strongest, but t the poorest graded, made the strongest concrete for the rich mix of the fixed proportional aggregates. For the practical mix as determined by Fuller's curve the Catalina stone gave concrete that was next to Eaton Canyon's in weakness. Therefore, it seems that in a rich mix a hard strong stone is more important than the grading of the stone, while in a practical field mix the grading of the stone is more important than the kind of stone.

 I_n the weathering test, where the stones were soaked in a concentrated sodium sulphate solution and then thoroughly dried, the San Gabriel stone proved to be far superior to the others. Therefore, where the concrete was to be subject to hard weathering, such as in a furnace or piers for a wharf, the San Gabriel stone would be chosen.

The specific gravities of the 3/4" stones were so close that the per cent voids were practically a direct function of the unit weights, the relation of course being inverse.

There were no tests made on the sands for silt, loam, or organic matter. In the first place all of the sands were washed at the plants, and second, they were brought to the Institute at different periods and were allowed to remain in the open some time before actual testing was started. These sands, unlike the stones, are known to approximately the same strengths, and since it is customary to purchase sand and rock from the same plant, little attention was given to the sands.

From a close inspection of the sands they were classified in the following order for weak and cleavable grains: Long Beach sand had the least, San Gabriel and San Fernando appeared to be in about the same class, and the Eaton Canyon sand had the most number of weak and cleavable grains. It is noted that the above grading of sands follows the classification of stones from a standpoint of texture and composition. The Eaton Canyon, San Gabriel, and San Fernando sands were angular to subangular, and the Long Beach was subrounded and therefore being least desireable in this respect for concrete. The results show that the Long Beach sand made the weakest cementsand specimens.

By comparing the unit weights of the sands and the fineness modulii of the sands it can be seen that the densities of the sands are about the same for San Fernando, San Gabriel, and Long Beach. The Eaton Canyon sand had a fair unit weight with a low fineness modulus which indicated a high density.

The cement-sand specimens were treated alike throughout the preparation, curing, and testing. The results indicated that the strength of these specimens was not directly related to the fineness modulus of the different sands. Mr. Newberry, who is in charge of the San Fernando district for Graham Bros. informed the writer that

the best fineness modulus for the particular sands here tested was from 2.85 to 3.00. San Gabriel and San Fernando were above this while Eaton Canyon and Long Beach were below this range. San Gabriel, which was closest to this range, had thegreatest unit strength for its cement-sand specimens.