CONCRETE AND CONCRETE AGGREGATES

THESIS

 by

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 $\mathcal{N}=\{1,\ldots,n\}$

INTRODUOTION

The primary purpose in choosing concrete and concrete aggregates for this thesis was to give the writer a more complete knowledge of these materials. especially for this immediate vicinity.

at the start of this experiment the fixed objeotive was to determine the relative strengths of some of the important stones that are located in Los Angeles and vicinity. This particular problem was brought to the writers attention after it was found that Eaton Canyon materials that were used in a recent addition to the Rose Bowl gave concrete that was far inferior to the concrete that was being prepared under the supervision of Mr. Byron Hill at the Institute, these latter materials being secured from San Gabriel. After some investigation the difference in stregth of these two concretes were found to be *1'* partially due to the aggregates.

After the experiment was started various tests and experiments somewhat naturally followed in succession. Some stones. for example. might be far superior in strength and hardness to stones from another source, yet the grading of these stones might be such as to give a relative weak concrete under some fixed mix. Or, as was also found, certain stones may be preferred under different conditions of use or preparation of the oonorete.

This report is on the whole somewhat interrelated and it would hardly be fair to quote too strongly any particular small section of the experiment.

The lack of consistency in certain phases of this experiment has left the writer with a dubious feeling as to the precise validity of oertatn parts. It is hoped that oertain parts *ot* this report shall encourage further work for a check on the results presented here and a continuation of a field of endeaver that has hardly been touched.

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Sand and stones were obtained from each of the following 4 sources; Eaton Canyon, San Fernando, San Gabriel, and Catalina. Long Beach. From this last source sand waa obtained at Long Beach and stones at Catalina. These 4 sources of materials were chosen because they were considered the most representative for Los Angeles, Pasadena, and vicinity. Concrete sand and both $3/4$ inch and $1\frac{1}{2}$ inch crushed rocks were desired. In some cases where $3/4$ inch and $1\frac{1}{2}$ inch rock could not be obtained, it was necessary to substitute l inch rock for the $3/4$ inch and 2 inch rock for the $1\frac{1}{2}$ inch rock. In some cases it was also necessary to substitute gravel for crushed rock.

The Eaton canyon sand and 1 inch crushed rock were obtained from a Pasadena City construction job, and the $l^{\frac{1}{2}}$ inch gravel was obtained from a bin at the Eaton Canyon plant.

San Fernando sand, 1 inch gravel, and 2 inch crushed rock were obtained from the bins at the San Fernando plant of Graham Bros.

San Gabriel sand, 1 inch gravel, and $1\frac{1}{2}$ inch gravel were obtained from the bins of Graham Bros.' San Gabriel plant.

The Long Beach sand and Catalina 1 and 2 inch crushed rocks were sent to the Institute through the courtesy of $R_$. W. Livingston who was at the time connected with Graham Bros., Inc.

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When all of the materials were on hand, a general geological observation was made with the assistance of David Scharf, a graduate geoldgist at the Institute.

The Eaton Canyon rock had a comparatively large amount of mica which was very cleavable. A good many of the rocks were gneissic, i.e., the minerals were arranged in bands or layers.

The Catalina rock was fine grained and compact with practically no bands or layers. The crushing surfaces were large and very smooth and flat. After several months of weathering the rocks seemed to become somewhat darker on the surfaces.

The San Fernando rock was somewhat less gneissic and micaceous than Eaton Canyon rock but there was, however, a noticible amount present. There were some decomposed peebbles present that could be broken with the hand.

The San Gabriel rock was finer grained and much less gneissic or micaceous than the rocks of Eaton Canyon or San Fernando. It conta ine d a few pebbles of decomposed rock.

The classification in strength, from a geological study of the rocks, is Catalina first, San Gabriel and San Fernando about the same, and Eaton Canyon by far the weakest.

PHYSICAL AND GEOLOGICAL SURVEY

3.

To carry the geological study of the rocks a little further, some rocks from each source were broken and the interior of the rocks were observed. The method of breaking the rocks gave an approximate relative measure of the strength of the four rocks. The poiture on page <u> β </u> illustrates the method of breaking. The same testing machine that was used for testing specimens was used but the maximum capacity was reduced to 15,000 pounds and the minimum scale division was then

one pound. The rate at which the cross-head moved was about .05 inches per minute, the same as is required for the standard compressive tests of concrete cylinders and which was used throughout. The individual rock that was **to** be tested was placed on a three quarter inch board that was laying on the weighing table and the load was applied through a steel bolt head that measured one and one quarter inches across and one half inch thick. The load was applied until the rock cracked, broke, or crushed. The board was used in order to give a large bearing area. on the bottom. The rocks that were to be tested had to be carefully selected to be sure that the load was applied by the bolt head on only one flat surface of contact and also that the bottom of the rock had a good resting area directly beneath the point where the load wae applied. Approximately eight pounds of ech of the four larger rocks were taken and from these the specimens were chosen. There were three rocks tested from each source as explained above and then three more were tested but instead of the three quarter inch board a $3/4$ inch square wood was used.

RESULTS OF BREAKING STRENGTH WITH 3/4 INCH

BOARD BASE

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REPRESENTATIVE BREAKING VALUES

RESULTS OF BREAKING STRENGTH WITH 3/4 INCH

STRIP AS BASE

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REPRESENTATIVE BREAKING VALUES

strength.

Nox banding. Comparative freshness

of rock and absence of banding or

sebistosity accounts for high

Since there were few suitable specimens from the 8 lbs. of rock samples in each group, those first chosen and used with the flat board test gave better and more representative results as the range indicates.

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TESTING PHYSICAL STRENGTH

 \mathcal{S}_{\cdot}

OF STONES

UNIT WEIGHTS OF MATERIALS

sand (Dried)

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Sands arrayed by weight San Fernando San Gabriel Eaton Canyon Long Beach

Catalina

 $1 - 1/2$ INCH STONE

Eaton Canyon

San Fernando

San Gabriel

Catalina

MIREANSKA

Arrayed by weight San Gabriel Eaton Canyon San Fernando Catalina

The apparent specifc gravity of the stones from each of the four sources were determined by the standard A. S. T. M. method. Each of the four small groups selected by a representative method were first weighed dry, indicated in the following table by A. After 24 hours soaking in water they were again weighed after being blotted dry - B. The soaked rocks were then submerged in water and the net weight is indicated below by C.

ARRAYED SPECIFIC GRAVITIES

PER CENT VOIDS IN 3/4 INCH ROCK

ARRAYED ACCORDING TO GRADING BY VOIDS

San Gabriel San Fernando Eaton Canyon Catalina

DISINTEGRATION TEST

Six lagge rocks were shosen from each of the 4 sources at random. These stones were tested for disintegration by soaking them in a saturated solution of sodium sulphate for about 12 hours and then drying them at about 100 degrees centigrade for the same length of time. All rocks were kept in the same solution and oven and treated exactly the same.

The fault with this test is the personal element in judging the extent of disintegration. Some of the stones cracked in half, others disintegrated to various extents, while some were not even effected. After about 10 complete cycles, the following order gives the resistance to disintegration.

ARRAYED ACBORDING TO RESISTANCE TO WEATHERING.

 $12.$

San Gabriel Catalina San Fernando **Eaton Canyon**

STONES TESTED FOR DISINTEGRATION

SIEVE ANALYSIS OF MATERIAIS

Representative samples were secured by quartering about 1/2 cubic foot of each material. The sand was oven dried at about 100° C. The free moisture in the sand was measured but has been omitted because of its lack of importance.

3/4 Inch Stone Sieve Analysis

 $16 -$

 $\bar{\mathbf{x}}$

 $\frac{1}{2}$

 \overline{g} .

 $\hat{\sigma}$

 \lesssim $\frac{1}{6}$

FINENESS MODULUS OF SAND

FINENESS MODULUS $\frac{7 \times 100 - 5}{100}$

Eaton Canyon 3.59 San Fernando 4.27 San Gabriel 4.22 Long Beach 3.75

ARRAYED FINENESS MODULUS San Fernando 3.27 San Gabriel $3,22$ Long Beach 2.75 **Eaton Canyon** 2.59

* In this final grouping the values are unity ; or one; less than above. Since the 200 mesh is commonly omitted these latter units take this into consideration.

CEMENT

The cement that was used was secured from the various construction jobs that were in progress at the Institute during the time of the experiments. All of the cement, which was of the Riverside Portland brand, came from the same sources but in different shipments. To check the consistency in method of testing and strength of the cement, 4 sacks were tested. The neat cement mixtures were all thoroughly mixed by hand in a glazed pan that was approximately 12 inches in diameter and 4 inches deep. For reference, the sacks were numbered. After the specimens were 1 day old they were then placed in the water curing tank in the testing materials laboratory. The temperature of the water in the curing tank was $139C$. Oiled metal forms were used for all the specimens. All compression tests were nade on the Tinius Olson Testing Machine with 150,000 lbs. maximum capacity and a minimum scale division of 10 lbe.

CEMENT MIXTURES

Grace Wt. Comont

Sack #1

• 2.86 gal. per cu. ft. Cement.

 12.41 The

CEMENT MIXTURES (CON.)

28 DAY CEMENT TESTS

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 $\sigma_{\rm{max}}=0.5$

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 $23.$

 $\label{eq:1} \omega^{(n)} = \frac{1}{\omega_n}$

Exactly 1000 c.c. (2.2 Lbs.) of water and 4.7 lbs. (0.05 cu. ft.) of cement were added to 0.10 cu. ft. of each of the 4 sands. All sands were room dried. When the specimens were 27 days old they were removed from Institute Laboratory curing tank. The specimens were tested from 18 to 23 hours after being removed from the curing tank. The temperature of the curing water was approximately 130 C. during the curing period. Since all metal forms were used, the folliwung areas represent the differmet specimens: the $1\frac{1}{2}$ " x 3" molds area . 2.01; the 2" cubes area = 4.0 ; the 3" x 6" cylinders area = 7.09; and unity for the briquettes.

EATON CANYON

SAN FERNANDO

 $\frac{1}{2} - \frac{1}{2}$

 $\int_0^{\infty} v(x)$

 $25 -$

SAN GABRIEL

 $6,600$ Median

LONG BEACH

RESULTS OF COMPRESSIVE TESTS FOR

CEMENT-SAND SPECIMENS

(Cylinders and cubes weighed alike)

ARRAYED SAND TEST RESULTS

San Gabriel Eaton Canyon San Fernando

Long Beach

CONCRETE SPECIMENS

Two series of cylinders were prepared and tested. One of the series was where the aggregate ratios were fixed, and the other was where the aggregate ratios for each set of materials were determined by use of Fuller's Curve for maximum density.

MIXES WITH FIXED RATIOS

For the mixes with the fixed ratios an attempt was made to make the break go through the rock, the purpose being to get the relative strength of the rooks. For this reason **a. rocky** mix was chosen. The water, cement, silt, and sand were constant for all of the mixes. San Gabriel silt and sand were used. The silt was the only aggregate with water in it. Since this was such a small quanity and constant for all mixes, it was neglected.

All concrete for the fixed ratio specimens were mixed by hand in the Institute Testing Laboratory. The mixing was done in a large metal mixing box, which was cleaned and dampened for each individual mix. The materials were first thoroughly mixed dry and then the water was carefully added and again the mixture was thoroughly mixed.

The specimens were numbered for each set. Number one was where the specimen form was of cardboard with a tin bottom. The cardboard had only a very light parafin coat. Number two, three, and four were cardboard forms that were well parafined throughout. Number two had a machined metal plate placed in its bottom to save later capping on

one end. Specimen number five was of metal It was, immediately after preparation, placed in the Ro-Tap Machine that is used for shaking sieves for twenty two complete shakes or about ten seconds. About one half inch of concrete was shaken out of the form. The water lost or brought to the top was negligable.

WT. PER CUBIC FOOT OF CONSTANTS

3/4 INCH STONES

a, P

MIXES WITH BOTH SIZES OF STOMES

WT. PER CUBIC FOOT OF CONSTANTS

3/4 INCH STONES

1-1/2 INCH STONES

Mix (by volume) 1.0 : 0.50 : 1.03 : 1.0 : 2.0 $W/G = 6.29$ U.S. gal. per cu. Ft. cement.

28 DAY COMPRESSION TEST

San Gabriel

$3/4$ " Stone

- 61.200 Break thru very few rock. $1.$
- 60,300 Very few breaks. Decomposition. $2.$
- 56,400 Practically no rock breaks. $3.$
- 55,800 ø 铀 $4.$ Ħ
- 63,200 Few Breaks. 5.

$3/4$ ^{*} and 1-1/2^{*} Stone

Less than 20,000 Lbs. Poor Matrix. Cement washed out. $\mathbf{1.}$

28 DAY COMPRESSION TEST (CON.)

$3/4$ ^{*} and $1-1/2$ ^{*} Stone

2. Less than 20,000 Lbs.

3. Less than 20,000. Hocks loose. No. rock breaks. Poor matrix.

Note: Poor bond thru-out.

Eaton Canyon $3/4$ ^{*} Stone

- 1. 62,500 Many rock breaks. Much mica exposed.
- 51,000 Poor bearing surface. 2.5
- $3.$ 54,700 Break thru several rocks.
- 66,100 Break thru still fewer rocks. $4.$
- 5. 58,900 Many rock breaks. Much mica exposed.

 $3/4^n$ and $1-1/2^n$ Stone

Note: Many rocks broken.

San Fernando

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$3/4$ Stone

- 1. 57,700 Break thru few rocks. Mica and decomposition.
- 2. 62,700 Same.

ŧê

- $3.65,000$
- $4.64,500$ 锑
- W 5. 65,700

28 DAY COMPRESSION TEST (CON.)

San Fernando

$3/4$ ⁿ and 1-1/2ⁿ Stone

- 1. 47,900 Break thru few. Mica and decomposition.
- 2. 59,000 Break thru few. Mica only.
- 3. 39,500 Break thru several. Note: Mostly mica. Some coarse \mathbb{X} . grained rocks ..

No tei Poor bond, Very few rock breaks.

Catalina

3/4" Stone

- 1. 65,500 Break thru several rocks. Rocks showed decomposition. 2, 73,200 Few breaks. Mostly decomposed rocks •.
- 3. 70,000 Break thru many rocks. While rock looks strong and fine grained and light in color, the breaks were dark and coarse grained, Few of the breaks looked strong like the exterior.

4. 69.600 Break thru several rocks. Rock showed decomposition. 5. 61.000 Break thru few rocks. About 1/2 decomposed. Note: A. specimen cracked at damp top at 50,000. Broken upper ha lf.

$3/4$ " and $1-1/2$ " Stone

32.

1. 43.400 Broke thru several big rocks and several small ones. Some of the big rocks appeared decomposed on the interior only. Note: Cracked at 42,000.

28 DAY COMPRESSION TEST (CON.)

Catalina

$3/4$ ⁿ and $1 - 1/2$ ⁿ Stone

39,200 Broke thru and around several large rocks. Eroke thru 2_o few small ones.

36,300 Broke thru several big rocks. Some of the broken rocks $3.$ looked strong while most of the breaks showed decomposition.

RESULTS OF 28 DAY COMPRESSION TESTS

FOR MIXES WITH WATER, CEMENT, AND SANDS FIXED.

$3/4$ " and $1-1/2$ " Stone Specimens.

ORDER IN MAGNITUDE OF STRENGTH

FOR SPECIMENS WITH 3/4" STONES.

ORDER IN MAGNITUDE OF STRENGTH

ORDER IN MAGNITUDE OF TESTED SPECIMENS

WITH GREATEST NUMBER OF BROKEN STONES.

Eaton Campon

Catalina

San Fernando - San Gabriel

35.

ORDER IN MAGNITUDE OF TESTED SPECIMENS

WITH BEST STONE BOND.

Eaton Canyon

Catalina $(3/4ⁿ)$

San Fernando

San Gabriel

MIXES DETERMINED BY FULLER'S CURVES

For the mixes determined by Fullers' Curves an attempt was made to get the most practical mix for the field with each of the four sets of materials. The water and cement were kept constant for all mixes.

All of the concrete in this test was mixed by means of a small gasoline mixer, the same as is very commonly used on small construction work. The drum was three feet in diameter and had three sets of paddles for mixing. It revolved at a rate of about eighteen revolutions per minute, its peripheral speed being about 170 feet per minute.All mixes were mixed for two minutes. The concrete was dumped from the mixer into a wheelbarrow. The concrete was placed in the specimen forms according to $A \bullet S \bullet T \bullet M$. One man placed concrete by thirds in each of the forms while a second man followed up with the tamping.

The day after preparation the specimens were stripped. The specimens were prepared and cured on the Institute campus beneath one of the large oak trees. The \texttt{IMX} three days following the preparation of the specimens they were lightly sprinkled each afternoon.

The following. mixes were finally decided upon as the most practical. These mixes were chosen after much labor and with the assistance of Byron Hill who was at the time connected with the Institute Civil Engineering Department.

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SPECIMENS PREPARED ACCORDING TO FULLER'S CURVES & METHODS

SHOWING A MIXER USED PLACE OF CURING FINISHED SPECIMENS

FULLER'S CURVES

San Fernando Materials

 $\bar{\mathbf{x}}$

 $\label{eq:1.1} \frac{\partial}{\partial t} \qquad \qquad \$

 $\mathbf{x} \in \mathbb{R}^n$

 $\frac{1}{2}$

 $\tilde{\mathcal{F}}^{(1)}$

FULLER'S CURVES

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FULLER'S CURVES

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FULLER'S CURVES

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RESULTS OF 28 DAY COMPRESSION TESTS

FOR MIXES DETERMINED BY FULLUR'S CURVES.

 $3/4$ " and $1-1/2$ " Stone Specimens.

TESTED SPECIMENS PREPARED ACCORDING TO FULLER'S CURVES

TABLE OF RESULTS

Notation: CatalinaCat. Long Beach............ San Gabriel.......S.G. San Fernando...... S.F. Eaton Canyon E.C.

Note: The water-cement ratio of all concrete specimens was 0.84 by volume.

Units used are pounds and inches.

Unit Strength of Concrete Specimens

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PRELIMINARY DISSCUSION.

The results given in this report should not be accepted too fully for several reasons. First, the results indicate that the accuracy was not very good. For example, in determining the unit weights of the sands with 2 different sized measuring devices there was a maximum variation of 3.6% . Incidentally, this variation was consistent throughout the 4 sets of measurements. Second, and most of important of all is the fact that the materials at the bins vary greatly from time to time. The writer made a sieve analysis of the San Gabriel $5/4$ " stone at 3 different times and the grading varied about as much as any 3 of the different $5/4$ " stones listed in this report. It can easily be seen how the grading can vary, even in a given bin. Likewise, but not to such an extent, there is the probability in variation in strength and other characteristics of a material from a given source. Because of these possible variations the results of such experiments as here recorded can only be fully accepted after a series of duplicate experiments had been conducted at different periods.

Throughout this experiment an attempt was made to secure only comparative values. For this reason the author has felt justified in diverging from common practice or the A.S.T.M. standards. For example; the finess moduluses here recorded are not standard because the Tyler standard set of sievs were not available, nevertheless the results are suitable for the purpose of this report. Then there are the empirical scales used for some of the curves, the purpose being to better illustrate the results. The water-cement ratio has been too high troughout and the curing ha sbeen very poor, yet the order and magnitude of the results are satisfactory except in a few instances were concrete was prepared with 1-1/2" stone.

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DISSOUSION.

The original test used for measuring the hardness of stones by crushing them individually on the testing machine is apparently a good method of measuring the hardness of stones. It would be interesting to check the results of a series of tests made on different stones by this method and by the standard rattler test. Obviously it is necessary to test 3 or more individual stones and use as the represenative value the median rather than the arithmetic average. Incidentally, it is interesting to note the almost negligible variation between the median and the arithmetic average for the breaking stresses of the concrete specimens. The median is favored over the arithmetic average because 1 freak specimen does not throw the final result off.

The primary object of this experiment was to measure the relative strengths of the different stones here presented, When the concrete specimens that were arbitrarily designed with fixed ratios were tested the stones were graded according to their number of broken surfaces and the bond to the matrix. The order in magnitude of broken stones depends upon the strength of the matrix and its adhesion to the stones and the strength of the stones. The adhesion of the matrix to the stones is dependent upon the porosity of the stones considering here dried stones) and the roughness of the surface of the stones. The line of failure in a concrete specimen may occur through the matrix, through the Stones, along the surface of the stones, or, as is most generally the case, through a combination of the above 3 possibilities

The specimens with the $1-1/2$ " stones gave low strengths because of the weakness of the matrix itself and the poor bond between the

بىمىتى

matrix and the stones. The water cement was the same as the water and cement for the specimens with the $3/4$ " stones. In the latter specimens. however, more water was used by the stones for absorption and the dampening of the surfaces' since there was more stone surfaces than where the $1-1/2$ " stones were used. The matrix was weak in the specimens with the large stone because the cement was washed out.

In the mixes with the smaller stone if the break occured along the surfaoes of the stones the expected order in strength for the oonorete specimens would be;l, Eaton Canyon: 2, Catalina; 3, San Fernando; 4, San Gabriel; this being the order of adhesion based on roughness. With regards to the surface the rooks may be classed in the following manner: Eaton Canyon rock was subangular stream gravel mixed with angular eruahed rook: Catalina was irregular crushed gravel with flat surfaces: San Fernando rock was sub-angular stream gravel: and the San Gabriel stone was sub-angular and sub-rounded stream gravel.

If the break was through the rock the $order$ given in the above paragraph would not necessarily hold but would probably hold if the stones were of the same strength and all other factors, except the smoothness of the surfaces.were the same. From actual observation the order for magnitude of number of broken stones in the tested specimens was: 1, Eaton Canyon; 2, Catalina; 3, San Fernando; 4, San Gabriel. This is the same order as the degree of surface roughness as given above; which indicates that the number of broken stones varies directly as the surface roughness of the stones.

A brief and complete summary of the geological sarvey of the stones is given on page $#3$ of this report.

It is interesting to note that the geological classification of the stones, which was made with no knowledge of the sources of the stones at the time, and the rock hardness test that was devised by

the writer as a substitute for the rattler test because of the lack of equipment, gave exactly the same results. Also, this same order was obtained from the concrete specimens that were prepared according to the fixed ratio of $1.0: 0.50: 1.03: 3.00$ by volume. Portland cement, San Gabriel silt, San Gabriel sand, and the $3/4$ ^H stones were used. The water-cement ratio was 0.84 by volume.

The Eaton Canyon stone proved to be the weakest in the hardness test and the poorest from a geological standpoint.\It also gave the weakest concrete in both the fixed ratio mixes and the mixes determined by Fuller's curves. Its specific gravity and its resistance to weathering were also the lowest. In the broken concrete speoimens the Eaton Canyon stones had more breaks than the stones of any other source. Obviously there is no question but what the Eaton Canyon stone is inferior to the others in all respects.

The Catalina stone, which was the hardest and strongest, but t the poorest graded, made the strongest concrete for the rich mix of the fixed proportional aggregates. For the practical mix as determined by Fuller's curve the Catalina stone gave concrete that was next to Eaton Canyon's in weakness. Therefore, it seems that in a rich mix a hard strong stone is more important than the grading of the stone, while in a practical field mix the grading of the stone is more important than the kind of stone.

 1_n the weathering test, where the stones were soaked in a concentrated sodium sulphate solution and then thoroughly dried, the San Gagriel stone proved to be far superior to the others. Therefore, where the concrete was to be subject to hard weathering; such as in a furnace or piers for a wharf, the San Gabriel stone would be ohosen.

The specific gravities of the $5/4$ " stones were so close that the per cent voids were practically a direct function of the unit weights, the relation of course being inverse.

There were no tests made on the sands for silt, loam, or organic matter. In the first place all of the sands were washed at the plants, and seoond, they were brought to the Institute at different periods and were allowed to remain in the open some time before actual tasting was started. These sands, unlike the stones, are known to approximately the same strengths, and since it is customary to purchase sand and rook from the same plant. little attention was given to the sands.

From a close inspeetion of the sands they were classified in the following order for weak and cleavable grains: Long Beach sand had the least, San Gabriel and San Fernando appeared to be in about the same class, and the Eaton Canyon sand had the most number of weak and cleavable grains. It is noted that the above grading of sands follows the classification of stones from a standpoint of texture and oomposi tion. The Eaton Canyon, Ban Gabriel• and san Fernando sands were angular to subangular, and the Long Beach was subrounded and therefore being least desireable in this respect for concrete. The results show that the Long Beach sand made the weakest cementsand specimens.

By comparing the unit weights of the sands and the fineness modulii of the sands it oan be seen that the densitiea of the sands are about the same for San Fernando, San Gabriel, and Long Beach. The Baton Canyon sand had a fair unit weight with a low fineness modulus whi oh indicated a high density.

The cement-sand specimens were treated alike throughout the preparation, ouring, and testing. The results indicated that the strength of these specimens was not directly relatedto the fineness modulus of the different sands. Mr. Newberry, who is in charge of the San Fernando district for Graham Bros. informed the writer that

the best fineness modulus for the particular sands here tested was from 2.85 to 3.00. San Gabriel and San Fernando were above this while Eaton Canyon and Long Beach were below this range. San Gabriel, which was closest to this range, had the greatest unit strength for its cement-sand specimens.